STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	SF-340020	1	14

REFERENCE: SF-340020

PROJECT: 17BP.5.R.69

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY *FRANKLIN*

PROJECT DESCRIPTION *BRIDGE NO. 20 ON SR 1114 (PEACH ORCHARD RD.) OVER CEDAR CREEK*

CONTENTS

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

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TRIGON

GOODNIGHT, D.J.

INVESTIGATED BY *DJG*

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SUBMITTED BY *FALCON ENG.*

DATE *OCTOBER 2017*

NORTH CAROLINA PROFESSIONAL SEAL 039779 ENGINEER JEREMY R. HAMM

10/11/17

SIGNATURE DATE

PROJECT REFERENCE NO.	SHEET NO.
SF-340020	2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION

SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,
VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6

SOIL LEGEND AND AASHTO CLASSIFICATION

GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)							SILT-CLAY MATERIALS (> 35% PASSING #200)				ORGANIC MATERIALS		
GROUP CLASS.	A-1		A-3	A-2				A-4	A-5	A-6	A-7	A-1, A-2 A-3	A-4, A-5 A-6, A-7	
	A-1-a	A-1-b		A-2-4	A-2-5	A-2-6	A-2-7				A-7-5, A-7-6			
SYMBOL														
% PASSING #10 #40 #200	50 MX 30 MX 15 MX	50 MX 25 MX	51 MN 10 MX	35 MX	35 MX	35 MX	35 MX	36 MN	36 MN	36 MN	36 MN	GRANULAR SOILS	SILT-CLAY SOILS	MUCK, PEAT
MATERIAL PASSING #40 LL PI	— 6 MX		— NP	40 MX 10 MX	41 MN 10 MX	40 MX 11 MN	41 MN 11 MN	40 MX 10 MX	41 MN 10 MX	40 MX 11 MN	41 MN 11 MN	SOILS WITH LITTLE OR MODERATE AMOUNTS OF ORGANIC MATTER		HIGHLY ORGANIC SOILS
GROUP INDEX	0		0	0		4 MX		8 MX	12 MX	16 MX	NO MX			
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS. GRAVEL, AND SAND		FINE SAND	SILTY OR CLAYEY GRAVEL AND SAND				SILTY SOILS		CLAYEY SOILS				
GEN. RATING AS SUBGRADE	EXCELLENT TO GOOD							FAIR TO POOR				FAIR TO POOR	POOR	UNSUITABLE

PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30

CONSISTENCY OR DENSENESS

PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTENCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH ($TONS/FT^2$)
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	< 4 4 TO 10 10 TO 30 30 TO 50 > 50	N/A
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30	< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4

TEXTURE OR GRAIN SIZE

U.S. STD. SIEVE SIZE	4	10	40	60	200	270
OPENING (MM)	4.76	2.00	0.42	0.25	0.075	0.053

BOULDER (BLDR.)	COBBLE (COB.)	GRAVEL (GR.)	COARSE SAND (CSE. SD.)	FINE SAND (F SD.)	SILT (SL.)	CLAY (CL.)

GRAIN SIZE	MM	305	75	2.0	0.25	0.05	0.005
	IN.	12	3				

SOIL MOISTURE - CORRELATION OF TERMS

SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION
	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
LL — LIQUID LIMIT		
PLASTIC RANGE (PI)	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
PL — PLASTIC LIMIT		
OM — OPTIMUM MOISTURE SL — SHRINKAGE LIMIT	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE
	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

PLASTICITY

	PLASTICITY INDEX (PI)	DRY STRENGTH
NON PLASTIC	0-5	VERY LOW
SLIGHTLY PLASTIC	6-15	SLIGHT
MODERATELY PLASTIC	16-25	MEDIUM
HIGHLY PLASTIC	26 OR MORE	HIGH

COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

GRADATION

WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.
UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.

ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.

COMPRESSIBILITY

SLIGHTLY COMPRESSIBLE	LL < 31
MODERATELY COMPRESSIBLE	LL = 31 - 50
HIGHLY COMPRESSIBLE	LL > 50

PERCENTAGE OF MATERIAL

ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL	
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE	1 - 10%
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE	10 - 20%
MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME	20 - 35%
HIGHLY ORGANIC	> 10%	> 20%	HIGHLY	35% AND ABOVE

GROUND WATER

- WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING
- STATIC WATER LEVEL AFTER 24 HOURS
- PW — PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA
- SPRING OR SEEP

MISCELLANEOUS SYMBOLS

- ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION
- SOIL SYMBOL
- ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT
- INFERRED SOIL BOUNDARY
- INFERRED ROCK LINE
- ALLUVIAL SOIL BOUNDARY
- 25/025 — DIP & DIP DIRECTION OF ROCK STRUCTURES
- SPT, DPT, DMT, VST, PMT — TEST BORING
- AUGER BORING
- CORE BORING
- MW — MONITORING WELL
- PIEZOMETER INSTALLATION
- SLOPE INDICATOR INSTALLATION
- CONE PENETROMETER TEST
- SOUNDING ROD
- TEST BORING WITH CORE
- SPT N-VALUE

RECOMMENDATION SYMBOLS

- UNDERCUT EXCAVATION
- SHALLOW UNDERCUT
- UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE
- UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK
- UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL

ABBREVIATIONS

AR - AUGER REFUSAL
BT - BORING TERMINATED
CL. - CLAY
CPT - CONE PENETRATION TEST
CSE. - COARSE
DMT - DILATOMETER TEST
DPT - DYNAMIC PENETRATION TEST
e - VOID RATIO
F - FINE
FOSS. - FOSSILIFEROUS
FRAC. - FRACTURED, FRACTURES
FRAGS. - FRAGMENTS
HI. - HIGHLY
MED. - MEDIUM
MICA. - MICACEOUS
MOD. - MODERATELY
NP - NON PLASTIC
ORG. - ORGANIC
PMT - PRESSUREMETER TEST
SAP. - SAPROLITIC
SD. - SAND, SANDY
SL. - SILT, SILTY
SLI. - SLIGHTLY
TCR - TRICONE REFUSAL
w - MOISTURE CONTENT
V - VERY
VST - VANE SHEAR TEST
WEA. - WEATHERED
γ - UNIT WEIGHT
γ_d - DRY UNIT WEIGHT

SAMPLE ABBREVIATIONS

S - BULK
SS - SPLIT SPOON
ST - SHELBY TUBE
RS - ROCK
RT - RECOMPACTED TRIAXIAL
CBR - CALIFORNIA BEARING RATIO

EQUIPMENT USED ON SUBJECT PROJECT

DRILL UNITS:
- [ ] CME-45C
- [ ] CME-55
- [ ] CME-550
- [ ] VANE SHEAR TEST
- [ ] PORTABLE HOIST
- [X] MOBILE B-57
- [ ] _____

ADVANCING TOOLS:
- [ ] CLAY BITS
- [ ] 6" CONTINUOUS FLIGHT AUGER
- [ ] 8" HOLLOW AUGERS
- [ ] HARD FACED FINGER BITS
- [ ] TUNG.-CARBIDE INSERTS
- [X] CASING [ ] W/ ADVANCER
- [X] TRICONE 2 15/16" STEEL TEETH
- [ ] TRICONE _____" TUNG.-CARB.
- [X] CORE BIT
- [ ] _____

HAMMER TYPE:
- [X] AUTOMATIC [ ] MANUAL

CORE SIZE:
- [ ] -B _____ [ ] -H _____
- [X] -N Q

HAND TOOLS:
- [ ] POST HOLE DIGGER
- [ ] HAND AUGER
- [ ] SOUNDING ROD
- [ ] VANE SHEAR TEST
- [ ] _____

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.
ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

WEATHERED ROCK (WR)	NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.
CRYSTALLINE ROCK (CR)	FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.
NON-CRYSTALLINE ROCK (NCR)	FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.
COASTAL PLAIN SEDIMENTARY ROCK (CP)	COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

WEATHERING

FRESH	ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.
VERY SLIGHT (V SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.
SLIGHT (SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.
MODERATE (MOD.)	SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.
MODERATELY SEVERE (MOD. SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. *IF TESTED, WOULD YIELD SPT REFUSAL*
SEVERE (SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF*
VERY SEVERE (V SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF*
COMPLETE	ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

ROCK HARDNESS

VERY HARD	CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.
HARD	CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.
MODERATELY HARD	CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.
MEDIUM HARD	CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.
SOFT	CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.
VERY SOFT	CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.

FRACTURE SPACING

TERM	SPACING
VERY WIDE	MORE THAN 10 FEET
WIDE	3 TO 10 FEET
MODERATELY CLOSE	1 TO 3 FEET
CLOSE	0.16 TO 1 FOOT
VERY CLOSE	LESS THAN 0.16 FEET

BEDDING

TERM	THICKNESS
VERY THICKLY BEDDED	4 FEET
THICKLY BEDDED	1.5 - 4 FEET
THINLY BEDDED	0.16 - 1.5 FEET
VERY THINLY BEDDED	0.03 - 0.16 FEET
THICKLY LAMINATED	0.008 - 0.03 FEET
THINLY LAMINATED	< 0.008 FEET

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.

FRIABLE	RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.
MODERATELY INDURATED	GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.
INDURATED	GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.
EXTREMELY INDURATED	SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.

AQUIFER - A WATER BEARING FORMATION OR STRATA.

ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.

ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.

ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.

CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.

COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.

CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.

DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.

DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.

FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.

FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.

FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.

FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.

FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.

JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.

LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.

LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.

MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.

PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.

RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.

ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.

SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.

SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.

STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.

STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.

STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.

TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK: BL-3:
N: 845037.5 E: 2182723.2
STA. 13+37.37 OFFSET: 14.3' LT, -L- ELEVATION: 252.79 FEET

NOTES:
FIAD - FILLED IMMEDIATELY AFTER DRILLING

DATE: 8-15-14

PROJECT REFERENCE NO.	SHEET NO.
SF–340020	2A

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000)

GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)

From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.

STRUCTURE

- INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities
- BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets
- VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets
- BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity
- DISINTEGRATED - poorly interlocked, heavily broken rock mass with mixture of angular and rounded rock pieces
- LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes

DECREASING INTERLOCKING OF ROCK PIECES ⇩

SURFACE CONDITIONS

VERY GOOD	GOOD	FAIR	POOR	VERY POOR
Very rough, fresh unweathered surfaces	Rough, slightly weathered, iron stained surfaces	Smooth, moderately weathered and altered surfaces	Slickensided, highly weathered surfaces with compact coatings or fillings or angular fragments	Slickensided, highly weathered surfaces with soft clay coatings or fillings

DECREASING SURFACE QUALITY ⇨

90			N/A	N/A
80				
70				
60				
50				
40				
30				
20				
10				
N/A	N/A			

AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)

GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos. P and Hoek E., 2000)

From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.

COMPOSITION AND STRUCTURE

- **A.** *Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.*
- **B.** *Sandstone with thin interlayers of siltstone*
- **C.** *Sandstone and siltstone in similar amounts*
- **D.** *Siltstone or silty shale with sandstone layers*
- **E.** *Weak siltstone or clayey shale with sandstone layers*
- **C, D, E,** and **G** - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to **F** and **H.**
- **F.** *Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure*
- **G.** *Undisturbed silty or clayey shale with or without a few very thin sandstone layers* → **H.** ***Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed into small rock pieces.***

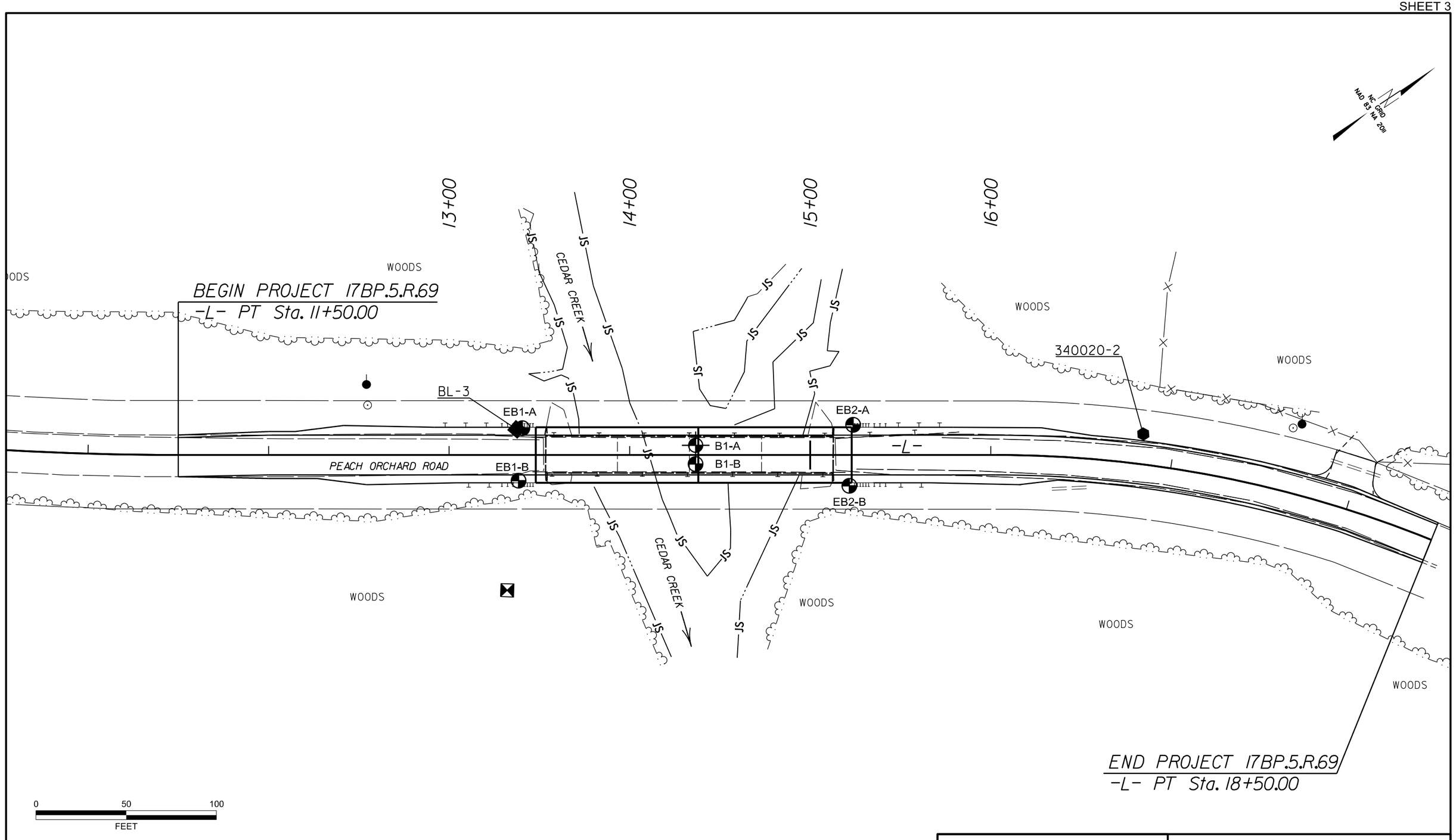
→ Means deformation after tectonic disturbance

SURFACE CONDITIONS OF DISCONTINUITIES (Predominantly bedding planes)

VERY GOOD	GOOD	FAIR	POOR	VERY POOR
Very Rough, fresh unweathered surfaces	Rough, slightly weathered surfaces	Smooth, moderately weathered and altered surfaces	Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments	Very smooth, slickensided or highly weathered surfaces with soft clay coatings or fillings

70	A			
60				
50		B	C	D E
40				
30				F
20				
10			G	H

DATE: 8-19-16



NOTES:

- PLANS ADOPTED FROM ELECTRONIC FILES RECEIVED FROM MOTT MACDONALD DATED DECEMBER 2016.
- BRIDGE SKEW: 90°

FALCON ENGINEERING
FALCON ENGINEERING, INC.
1210 TRINITY ROAD, SUITE 110
RALEIGH, NC 27607
PHONE: 919.871.0800
FAX: 919.871.0803

BORING LOCATION PLAN

BRIDGE NO. 20 ON SR 1114 (PEACH ORCHARD RD.)
OVER CEDAR CREEK
FRANKLIN COUNTY, NORTH CAROLINA
WBS NO.: 17BP.5.R.69 | TIP NO.: SF-340020
FALCON PROJECT NO. G16029.03

PROJECT REFERENCE	SHEET NO.
SF-340020	4
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

HATCH MOTT MACDONALD I & E, LLC LICENSE NO. F-0669 | ICA ENGINEERING LICENSE NO. F-0258

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

Prepared in the Office of:
MOTT MACDONALD — PO Box 700, Fuquay-Varina, NC 27526, www.mottmac.com
ICA Engineering — 5121 Kingsdom Way, Suite 100, Raleigh, NC 27607, NC License No. 76020

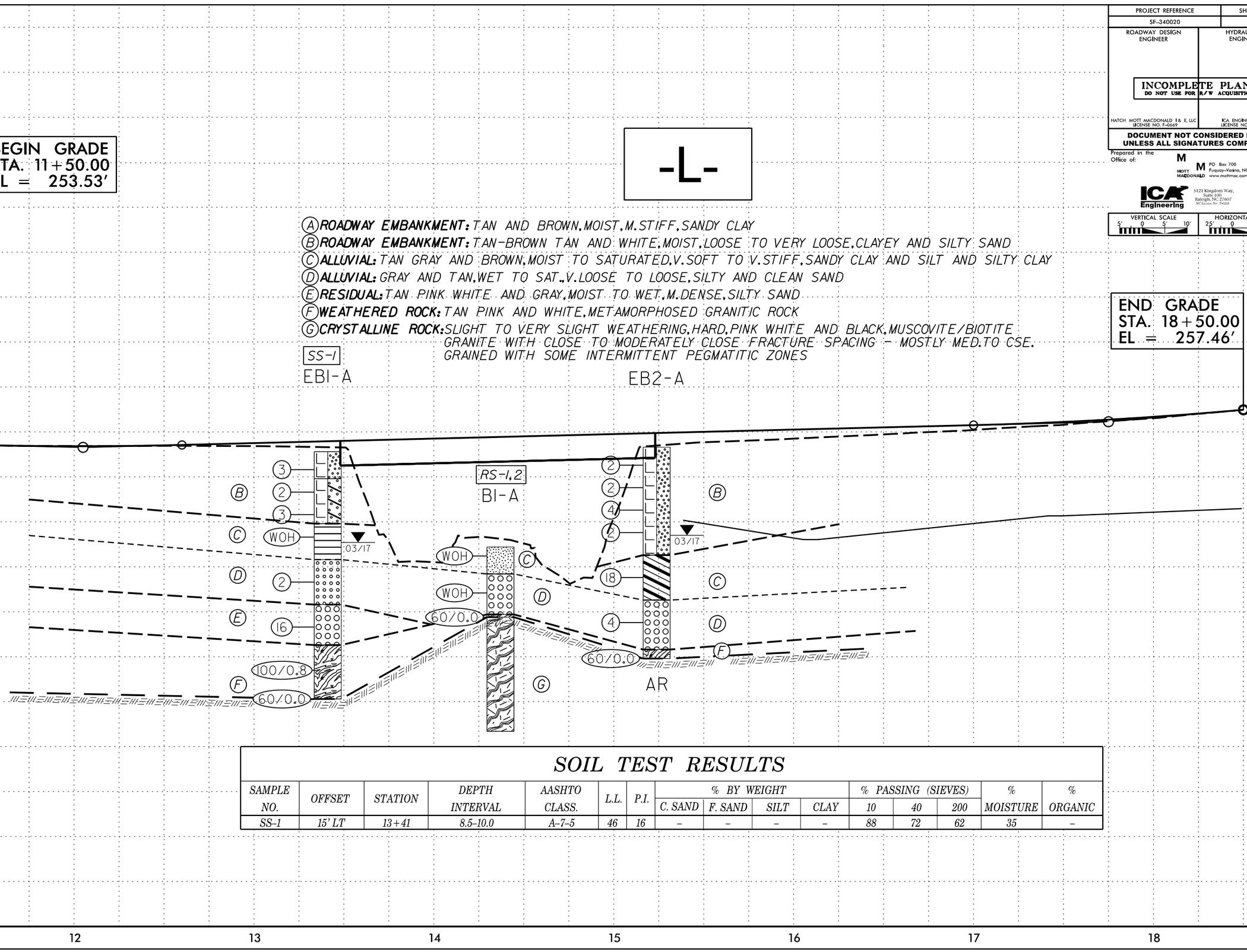
VERTICAL SCALE 5' 0 5' 10' | HORIZONTAL SCALE 25' 0 12.5' 25'

BEGIN GRADE
STA. 11+50.00
EL = 253.53'

END GRADE
STA. 18+50.00
EL = 257.46'

-L-

(A) **ROADWAY EMBANKMENT:** TAN AND BROWN, MOIST, M.STIFF, SANDY CLAY
(B) **ROADWAY EMBANKMENT:** TAN-BROWN TAN AND WHITE, MOIST, LOOSE TO VERY LOOSE, CLAYEY AND SILTY SAND
(C) **ALLUVIAL:** TAN GRAY AND BROWN, MOIST TO SATURATED, V.SOFT TO V.STIFF, SANDY CLAY AND SILT AND SILTY CLAY
(D) **ALLUVIAL:** GRAY AND TAN, WET TO SAT., V.LOOSE TO LOOSE, SILTY AND CLEAN SAND
(E) **RESIDUAL:** TAN PINK WHITE AND GRAY, MOIST TO WET, M.DENSE, SILTY SAND
(F) **WEATHERED ROCK:** TAN PINK AND WHITE, METAMORPHOSED GRANITIC ROCK
(G) **CRYSTALLINE ROCK:** SLIGHT TO VERY SLIGHT WEATHERING, HARD, PINK WHITE AND BLACK, MUSCOVITE/BIOTITE GRANITE WITH CLOSE TO MODERATELY CLOSE FRACTURE SPACING - MOSTLY MED.TO CSE. GRAINED WITH SOME INTERMITTENT PEGMATITIC ZONES



SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-1	15' LT	13+41	8.5-10.0	A-7-5	46	16	-	-	-	-	88	72	62	35	-

$$DATE$$ 09-OCT-2017 12:25
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cadmachine AT GEOT-10

EB1

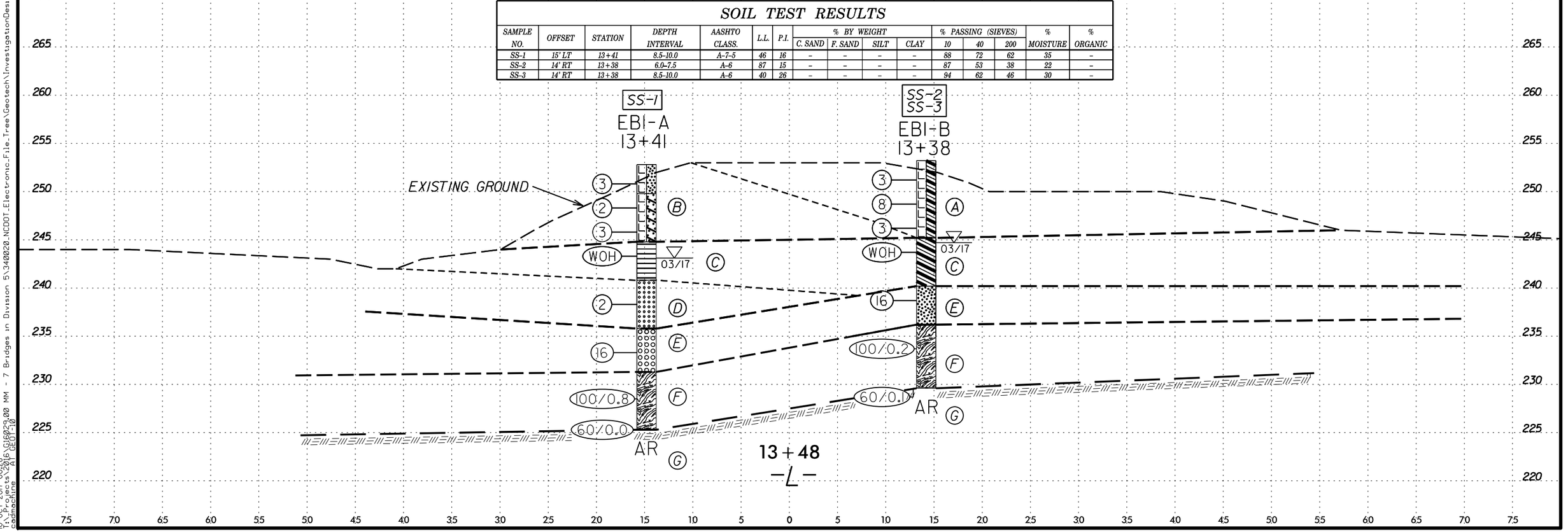
NOTES:

- *GROUNDLINE CROSS SECTION ALONG BENT LINE DRAWN FROM TOPOGRAPHIC DATA FROM ELECTRONIC FILES RECEIVED FROM MOTT MACDONALD DATED DECEMBER 2016.*
- *INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION.*
- *BRIDGE SKEW: 90 DEGREES*

(A) **ROADWAY EMBANKMENT:** TAN, MOIST, SOFT TO MED. STIFF, SILTY CLAY
(B) **ROADWAY EMBANKMENT:** TAN-BROWN AND TAN, MOIST, V. LOOSE, CLAYEY AND SILTY SAND
(C) **ALLUVIAL:** GRAY AND TAN, MOIST TO SAT., V. SOFT, SILTY SAND
(D) **ALLUVIAL:** GRAY, SAT., V. LOOSE, SILTY SAND
(E) **RESIDUAL:** TAN PINK WHITE AND GRAY, MOIST TO WET, M. DENSE, SILTY SAND
(F) **WEATHERED ROCK:** TAN, GRANITIC ROCK
(G) **CRYSTALLINE ROCK:** TAN, GRANITIC ROCK

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-1	15' LT	13+41	8.5-10.0	A-7-5	46	16	–	–	–	–	88	72	62	35	–
SS-2	14' RT	13+38	6.0-7.5	A-6	87	15	–	–	–	–	87	53	38	22	–
SS-3	14' RT	13+38	8.5-10.0	A-6	40	26	–	–	–	–	94	62	46	30	–

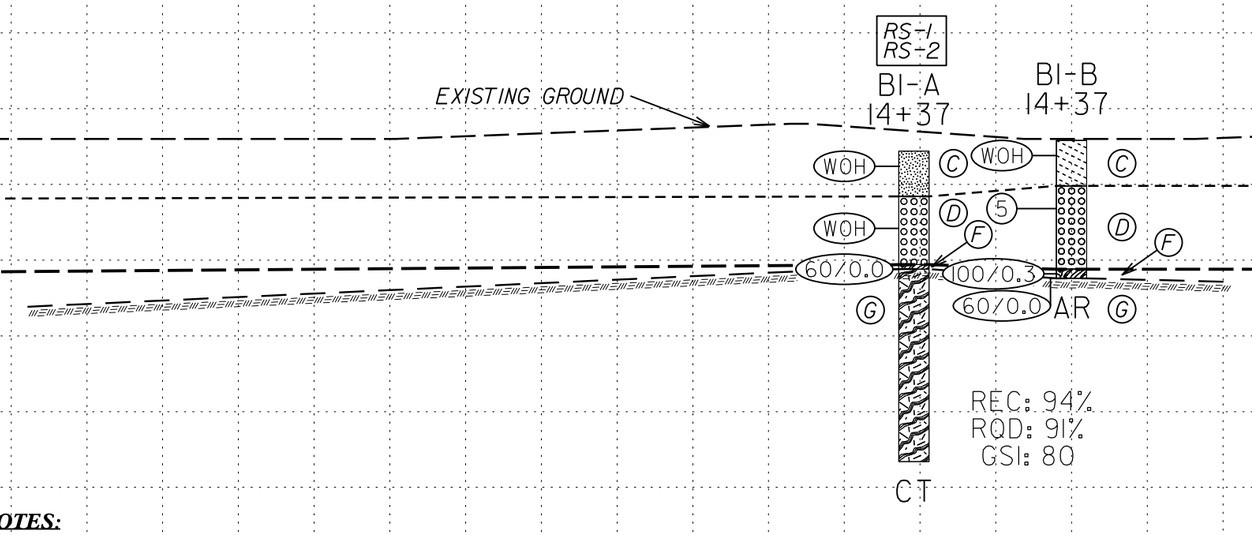


PROJ. REFERENCE NO.	SHEET NO.
SF-340020	6

0 2.5 5

B1

(C) **ALLUVIAL:** BROWN,WET,V.SOFT,SANDY SILT
(D) **ALLUVIAL:** TAN,SAT.,V.LOOSE TO LOOSE,CLEAN SAND
(F) **WEATHERED ROCK:** TAN,GRANITIC ROCK
(G) **CRYSTALLINE ROCK:** SLIGHT TO VERY SLIGHT WEATHERING,HARD,PINK WHITE AND BLACK, MUSCOVITE/BIOTITE GRANITE WITH CLOSE TO MODERATELY CLOSE FRACTURE SPACING - MOSTLY MED.TO CSE.GRAINED WITH SOME INTERMITTENT PEGMATITIC ZONES



NOTES:

- *GROUNDLINE CROSS SECTION ALONG BENT LINE DRAWN FROM TOPOGRAPHIC DATA FROM ELECTRONIC FILES RECEIVED FROM MOTT MACDONALD DATED DECEMBER 2016.*
- *INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION.*
- *BRIDGE SKEW: 90 DEGREES*

14+35.5

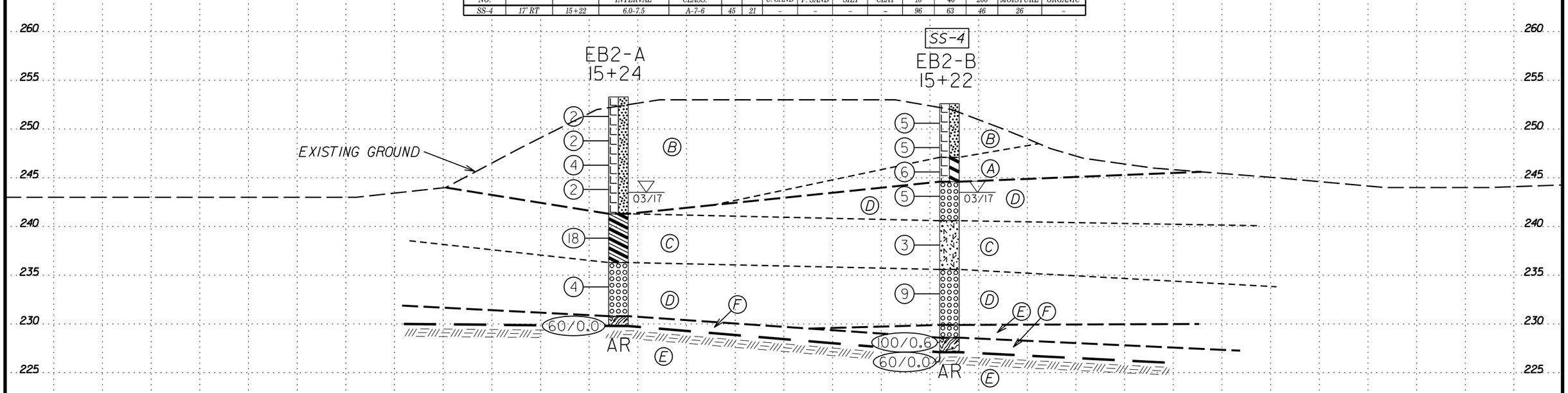
-L-

EB2

(A) **ROADWAY EMBANKMENT:** TAN AND BROWN, MOIST, MED. SOFT, SANDY CLAY
(B) **ROADWAY EMBANKMENT:** TAN-BROWN TAN AND WHITE, MOIST, V. LOOSE TO LOOSE, SILTY SAND
(C) **ALLUVIAL:** GRAY TAN-BROWN AND GRAY, MOIST TO SAT., SOFT TO HARD, SANDY CLAYEY SILT AND SANDY CLAY
(D) **ALLUVIAL:** TAN AND GRAY, WET TO SAT., V. LOOSE TO LOOSE, SILTY SAND
(E) **RESIDUAL:** TAN, WET, V. DENSE, SILTY SAND
(F) **WEATHERED ROCK:** TAN PINK AND WHITE, GRANITE
(G) **CRYSTALLINE ROCK:** TAN PINK AND WHITE, GRANITE

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-4	17' RT	15+22	6.0-7.5	A-7-6	45	21	–	–	–	–	96	63	46	26	–



NOTES:

- GROUNDLINE CROSS SECTION ALONG BENT LINE DRAWN FROM TOPOGRAPHIC DATA FROM ELECTRONIC FILES RECEIVED FROM MOTT MACDONALD DATED DECEMBER 2016.
- INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION.
- BRIDGE SKEW: 90 DEGREES

15+23

–L–

GEOTECHNICAL BORING REPORT
BORE LOG

WBS 17BP.5.R.69	**TIP** SF-340020	**COUNTY** ALAMANCE	**GEOLOGIST** Goodnight, D.	
SITE DESCRIPTION BRIDGE NO. 20 ON SR 1114 (PEACH ORCHARD RD.) OVER CEDAR CREEK				**GROUND WTR (ft)**
BORING NO. EB1-A	**STATION** 13+41	**OFFSET** 15 ft LT	**ALIGNMENT** -L-	**0 HR.** 9.7
COLLAR ELEV. 252.8 ft	**TOTAL DEPTH** 27.5 ft	**NORTHING** 845,041	**EASTING** 2,182,725	**24 HR.** 10.1
DRILL RIG/HAMMER EFF./DATE TRI8016 MOBILE B-57 97% 02/24/2017		**DRILL METHOD** H.S. Augers	**HAMMER TYPE** Automatic	
DRILLER Estep, J. E.	**START DATE** 03/29/17	**COMP. DATE** 03/29/17	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
										ELEV. (ft) DEPTH (ft)
255										
	252.8	0.0								252.8 0.0
	251.8	1.0	3	2	1	3		M		**ROADWAY EMBANKMENT** TAN-BROWN, SILTY CSE. TO F. SAND (A-2-4)
250	249.8	3.0								249.8 3.0
	249.3	3.5	1	1	1	2		M		**ROADWAY EMBANKMENT** TAN, CLAYEY SAND (A-2-6) WITH TRACE GRAVEL
	246.8	6.0	1	1	2	3		M		
245	244.8	8.0								244.8 8.0
	244.3	8.5	1	1	WOH	1	SS-1	35%		**ALLUVIAL** GRAY, SILTY CLAY (A-7-5) WITH LITTLE ORGANICS (WOOD FRAGS.)
240	240.8	12.0								240.8 12.0
	239.3	13.5	1	1	1	2		Sat.		GRAY, SLIGHTLY SILTY F. SAND (A-3) WITH TRACE ORGANICS
235	235.8	17.0								235.8 17.0
	234.3	18.5	7	8	8	16		W		**RESIDUAL** TAN AND PINK, SLIGHTLY SILTY F. TO CSE. SAND (A-1-b)
230	231.3	21.5								231.3 21.5
	229.3	23.5	42	58/0.3		100/0.8				**WEATHERED ROCK** TAN, GRANITE
	225.3	27.5	60/0.0			60/0.0				225.3 27.5
										Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 225.3 ft ON CR: GRANITE

WBS 17BP.5.R.69	**TIP** SF-340020	**COUNTY** ALAMANCE	**GEOLOGIST** Goodnight, D.	
SITE DESCRIPTION BRIDGE NO. 20 ON SR 1114 (PEACH ORCHARD RD.) OVER CEDAR CREEK				**GROUND WTR (ft)**
BORING NO. EB1-B	**STATION** 13+38	**OFFSET** 14 ft RT	**ALIGNMENT** -L-	**0 HR.** 2.0
COLLAR ELEV. 253.2 ft	**TOTAL DEPTH** 23.6 ft	**NORTHING** 845,023	**EASTING** 2,182,748	**24 HR.** 8.5
DRILL RIG/HAMMER EFF./DATE TRI8016 MOBILE B-57 97% 02/24/2017		**DRILL METHOD** H.S. Augers	**HAMMER TYPE** Automatic	
DRILLER Estep, J. E.	**START DATE** 03/29/17	**COMP. DATE** 03/29/17	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
255										
	253.2	0.0								253.2 0.0
	252.2	1.0	2	2	1	3		M		**ROADWAY EMBANKMENT** TAN, SILTY SAND (A-6) WITH TRACE GRAVEL
250	249.7	3.5	2	4	4	8		M		
	247.2	6.0	2	1	2	3	SS-2	22%		
245	245.2	8.0								245.2 8.0
	244.7	8.5	2	1	WOH	1	SS-3	30%		**ALLUVIAL** TAN AND GRAY, SILTY SAND (A-6)
240	240.2	13.0								240.2 13.0
	239.7	13.5	4	6	10	16		M		**RESIDUAL** GRAY AND WHITE, SILTY SAND (A-2-4) SAPROLITIC
235	236.2	17.0								236.2 17.0
	234.7	18.5	100/0.2			100/0.2				**WEATHERED ROCK** TAN, GRANITE
230	229.7	23.5	60/0.1			60/0.1				229.7 23.5
	229.6	23.6								**CRYSTALLINE ROCK** TAN, GRANITE 229.6 23.6
										Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 229.6 ft IN CR: GRANITE

GEOTECHNICAL BORING REPORT
BORE LOG

WBS 17BP.5.R.69	**TIP** SF-340020	**COUNTY** ALAMANCE	**GEOLOGIST** Goodnight, D.	
SITE DESCRIPTION BRIDGE NO. 20 ON SR 1114 (PEACH ORCHARD RD.) OVER CEDAR CREEK				**GROUND WTR (ft)**
BORING NO. B1-A	**STATION** 14+37	**OFFSET** 5 ft LT	**ALIGNMENT** -L-	**0 HR.** 0.8
COLLAR ELEV. 242.2 ft	**TOTAL DEPTH** 20.5 ft	**NORTHING** 845,115	**EASTING** 2,182,786	**24 HR.** FIAD
DRILL RIG/HAMMER EFF./DATE TRI8016 MOBILE B-57 97% 02/24/2017		**DRILL METHOD** H.S. Augers	**HAMMER TYPE** Automatic	
DRILLER Estep, J. E.	**START DATE** 03/30/17	**COMP. DATE** 03/30/17	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
										ELEV. (ft) DEPTH (ft)
245										
	242.2	0.0	WOH	WOH	WOH	0.		W		242.2 0.0 **ALLUVIAL** BROWN, SANDY SILT (A-4) WITH LENSES OF CSE. SAND
240										239.2 3.0 TAN, F. TO CSE. SAND (A-1-b)
	238.1	4.1	WOH	WOH	WOH	0.		Sat.		
235	234.4	7.8	60/0.0			60/0.0				234.7 7.5 234.4 7.8 **WEATHERED ROCK** TAN, GRANITE
230							RS-1			**CRYSTALLINE ROCK** SLIGHT TO VERY SLIGHT WEATHERING, HARD, PINK WHITE AND BLACK, MUSCOVITE/BIOTITE GRANITE WITH CLOSE TO MODERATELY CLOSE FRACTURE SPACING - MOSTLY MED. TO CSE. GRAINED WITH SOME INTERMITTENT PEGMATITIC ZONES
225							RS-2			221.7 REC = 94%, RQD = 91%, GSI = 80 20.5 Boring Terminated at Elevation 221.7 ft IN CR: GRANITE

GEOTECHNICAL BORING REPORT
CORE LOG

WBS 17BP.5.R.69	**TIP** SF-340020	**COUNTY** ALAMANCE	**GEOLOGIST** Goodnight, D.	
SITE DESCRIPTION BRIDGE NO. 20 ON SR 1114 (PEACH ORCHARD RD.) OVER CEDAR CREEK				**GROUND WTR (ft)**
BORING NO. B1-A	**STATION** 14+37	**OFFSET** 5 ft LT	**ALIGNMENT** -L-	**0 HR.** 0.8
COLLAR ELEV. 242.2 ft	**TOTAL DEPTH** 20.5 ft	**NORTHING** 845,115	**EASTING** 2,182,786	**24 HR.** FIAD
DRILL RIG/HAMMER EFF./DATE TRI8016 MOBILE B-57 97% 02/24/2017		**DRILL METHOD** H.S. Augers	**HAMMER TYPE** Automatic	
DRILLER Estep, J. E.	**START DATE** 03/30/17	**COMP. DATE** 03/30/17	**SURFACE WATER DEPTH** N/A	
CORE SIZE NQ	**TOTAL RUN** 12.7 ft			

ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (ft) %	RUN RQD (ft) %	SAMP. NO.	STRATA REC. (ft) %	STRATA RQD (ft) %	LOG	DESCRIPTION AND REMARKS
											ELEV. (ft) DEPTH (ft)
234.4											Begin Coring @ 7.8 ft
	234.4	7.8	2.7	3:01/0.7 4:34/1.0 5:57/1.0	(2.5) 93%	(2.4) 89%		(12.0) 94%	(11.5) 91%		234.4 **CRYSTALLINE ROCK** 7.8 SLIGHT TO VERY SLIGHT WEATHERING, HARD, PINK WHITE AND BLACK, MUSCOVITE/BIOTITE GRANITE WITH CLOSE TO MODERATELY CLOSE FRACTURE SPACING - MOSTLY MED. TO CSE. GRAINED WITH SOME INTERMITTENT PEGMATITIC ZONES
	231.7	10.5	5.0	5:41/1.0 6:23/1.0 8:25/1.0 9:37/1.0 8:59/1.0	(5.0) 100%	(5.0) 100%	RS-1				REC = 94%, RQD = 91%, GSI = 80
230											
	226.7	15.5	5.0	15:30/1.0 6:24/1.0 3:54/1.0 5:58/1.0 5:11/1.0	(4.5) 90%	(4.1) 82%	RS-2				
225											
	221.7	20.5									221.7 20.5 Boring Terminated at Elevation 221.7 ft IN CR: GRANITE

BORING B1-A, BOX 1 OF 2, 7.8 FEET TO 15.5 FEET.

BORING B1-A, BOX 2 OF 2, 15.5 FEET TO 20.5 FEET.

0 4" 8"
INCHES

FALCON ENGINEERING, INC.
1210 TRINITY ROAD, SUITE 110
RALEIGH, NC 27607
PHONE: 919.871.0800
FAX: 919.871.0803

ROCK CORE PHOTOGRAPHS

BRIDGE NO. 20 ON SR 1114 (PEACH ORCHARD RD.)
OVER CEDAR CREEK
FRANKLIN COUNTY, NORTH CAROLINA
WBS NO.: 17BP.5.R.69 | TIP NO.: SF-340020
FALCON PROJECT NO. G16029.03

GEOTECHNICAL BORING REPORT
BORE LOG

WBS 17BP.5.R.69	**TIP** SF-340020	**COUNTY** ALAMANCE	**GEOLOGIST** Goodnight, D.	
SITE DESCRIPTION BRIDGE NO. 20 ON SR 1114 (PEACH ORCHARD RD.) OVER CEDAR CREEK				**GROUND WTR (ft)**
BORING NO. B1-B	**STATION** 14+37	**OFFSET** 5 ft RT	**ALIGNMENT** -L-	**0 HR.** 1.1
COLLAR ELEV. 242.9 ft	**TOTAL DEPTH** 9.1 ft	**NORTHING** 845,109	**EASTING** 2,182,794	**24 HR.** FIAD
DRILL RIG/HAMMER EFF./DATE TRI8016 MOBILE B-57 97% 02/24/2017		**DRILL METHOD** H.S. Augers	**HAMMER TYPE** Automatic	
DRILLER Estep, J. E.	**START DATE** 03/30/17	**COMP. DATE** 03/30/17	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
245										
	242.9	0.0	WOH	WOH	WOH	0		W		**ALLUVIAL** BROWN, F. SANDY SILT (A-4) WITH LENSES OF CLEAN CSE. SAND AND LITTLE ORGANICS TAN, F. TO CSE. SAND (A-1-b)
240	239.4	3.5	2	2	3	5		Sat.		
	234.4	8.5	100/0.3			100/0.3				**WEATHERED ROCK** TAN, GRANITE Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 233.8 ft ON CR: GRANITE
235	233.8	9.1	60/0.0			60/0.0				

242.9 0.0
239.9 3.0
234.4 8.5
233.8 9.1

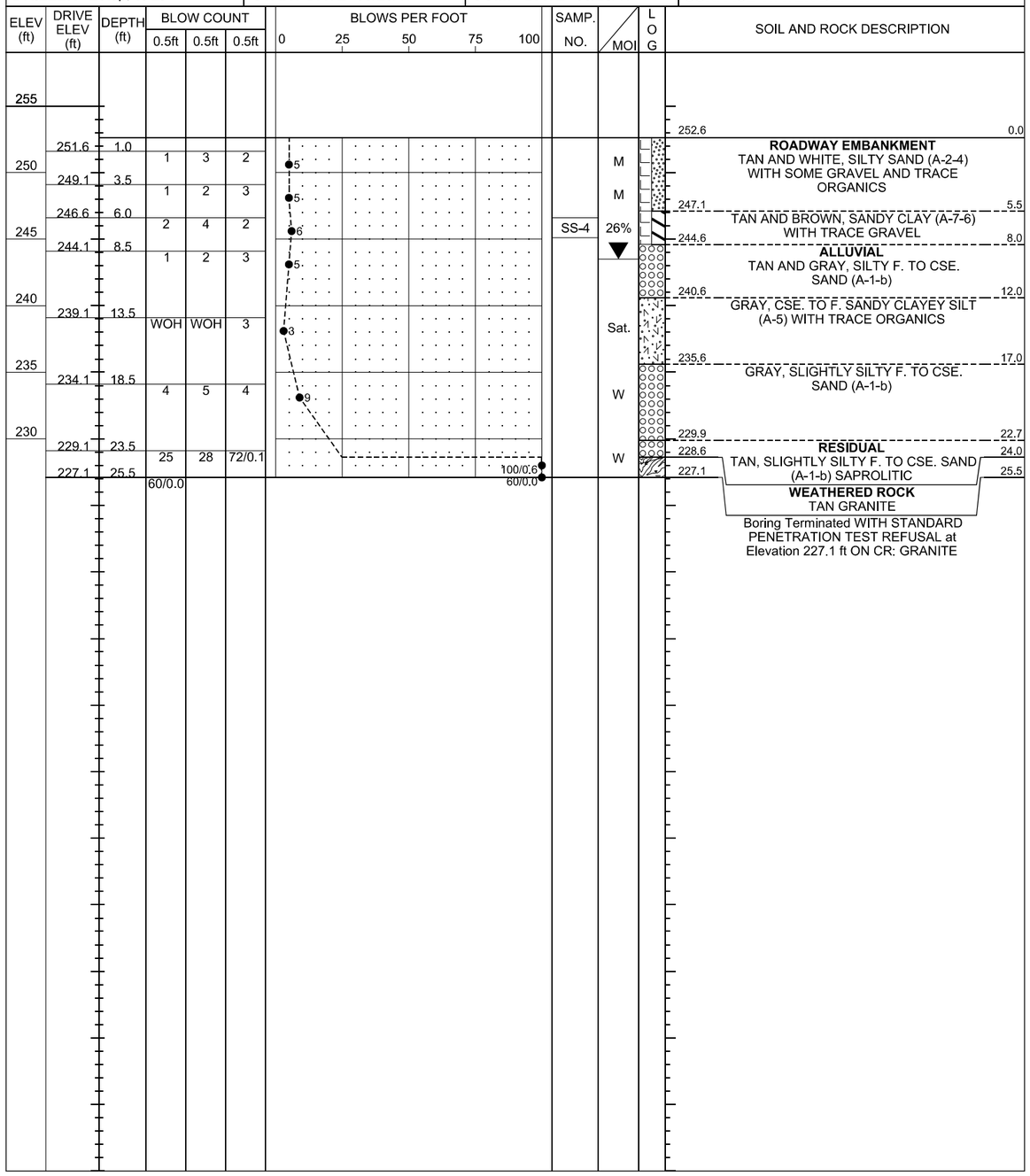
WBS 17BP.5.R.69	**TIP** SF-340020	**COUNTY** ALAMANCE	**GEOLOGIST** Goodnight, D.	
SITE DESCRIPTION BRIDGE NO. 20 ON SR 1114 (PEACH ORCHARD RD.) OVER CEDAR CREEK				**GROUND WTR (ft)**
BORING NO. EB2-A	**STATION** 15+24	**OFFSET** 17 ft LT	**ALIGNMENT** -L-	**0 HR.** 12.8
COLLAR ELEV. 253.3 ft	**TOTAL DEPTH** 23.5 ft	**NORTHING** 845,194	**EASTING** 2,182,825	**24 HR.** 9.8
DRILL RIG/HAMMER EFF./DATE TRI8016 MOBILE B-57 97% 02/24/2017		**DRILL METHOD** H.S. Augers	**HAMMER TYPE** Automatic	
DRILLER Estep, J. E.	**START DATE** 03/28/17	**COMP. DATE** 03/28/17	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION ELEV. (ft) DEPTH (ft)
255										
	253.3	0.0								**ROADWAY EMBANKMENT** TAN-BROWN SILTY SAND (A-2-4) WITH TRACE GRAVEL AND LITTLE MICA
	252.3	1.0	1	1	1	2		M		
250	249.8	3.5	1	1	1	2		M		
	247.3	6.0	2	2	2	4		M		
245	244.8	8.5	1	1	1	2		▼		
	241.3	12.0								**ALLUVIAL** TAN-BROWN AND GRAY, SANDY CLAY (A-6)
240	239.8	13.5	6	9	9	18		M		
	236.3	17.0								TAN AND GRAY, SLIGHTLY SILTY F. TO CSE. SAND (A-1-b) WITH TRACE GRAVEL
235	234.8	18.5	2	2	2	4		Sat.		
	230.8	22.5								**WEATHERED ROCK** PINK AND WHITE GRANITE Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 229.8 ft ON CR: GRANITE
230	229.8	23.5	60/0.0			60/0.0				

253.3 0.0
241.3 12.0
236.3 17.0
230.8 22.5
229.8 23.5

GEOTECHNICAL BORING REPORT
BORE LOG

WBS 17BP.5.R.69	**TIP** SF-340020	**COUNTY** ALAMANCE	**GEOLOGIST** Goodnight, D.	
SITE DESCRIPTION BRIDGE NO. 20 ON SR 1114 (PEACH ORCHARD RD.) OVER CEDAR CREEK				**GROUND WTR (ft)**
BORING NO. EB2-B	**STATION** 15+22	**OFFSET** 17 ft RT	**ALIGNMENT** -L-	**0 HR.** 9.0
COLLAR ELEV. 252.6 ft	**TOTAL DEPTH** 25.5 ft	**NORTHING** 845,174	**EASTING** 2,182,852	**24 HR.** 9.1
DRILL RIG/HAMMER EFF./DATE TRI8016 MOBILE B-57 97% 02/24/2017		**DRILL METHOD** H.S. Augers		**HAMMER TYPE** Automatic
DRILLER Estep, J. E.	**START DATE** 03/28/17	**COMP. DATE** 03/28/17	**SURFACE WATER DEPTH** N/A	



LABORATORY TEST RESULTS

Bridge No. 20 over Cedar Cr. on SR 1114
Franklin County, North Carolina
Falcon Engineering Project Number: G16029.03
June 8, 2017

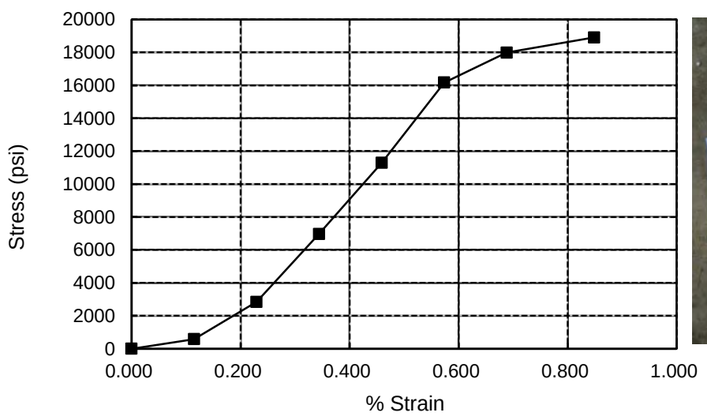
UNIAXIAL COMPRESSIVE STRENGTH OF INTACT ROCK CORE SPECIMENS *Performed in General Accordance with ASTM D7012*

Sample ID.: RS-1
Location: B1-A
Depth (ft): 10.5-11.0

Length (in.): 4.36
Diameter (in.): 1.98
Area (in²): 3.079
L/D 2.20
Unit Weight (pcf): 161.5

Compressive Strength (psi): 18910
Time to Failure, mins:sec: 10:46

Deflection (in.)	Strain (%)	Load (lbf)	Compressive Strength (psi)	Young's Modulus (psi)
0.000	0.000	0	0	
0.005	0.115	1780	580	505,760
0.010	0.229	8740	2840	1,238,240
0.015	0.344	21470	6970	2,025,947
0.020	0.459	34750	11290	2,461,220
0.025	0.573	49780	16170	2,820,048
0.030	0.688	55400	17990	2,614,547
0.037	0.849	58220	18910	2,228,314

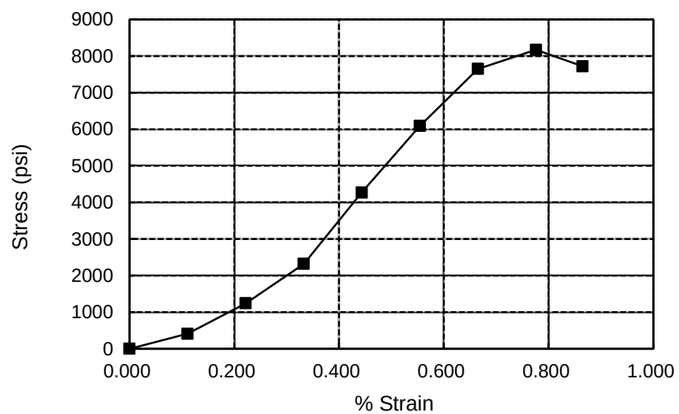


Sample ID.: RS-2
Location: B1-A
Depth (ft): 16.5-17.0

Length (in.): 4.51
Diameter (in.): 1.98
Area (in²): 3.079
L/D 2.28
Unit Weight (pcf): 162.9

Compressive Strength (psi): 8170
Time to Failure, mins:sec: 8:25

Deflection (in.)	Strain (%)	Load (lbf)	Compressive Strength (psi)	Young's Modulus (psi)
0.000	0.000	0	0	
0.005	0.111	1260	410	369,820
0.010	0.222	3810	1240	559,240
0.015	0.333	7150	2320	697,547
0.020	0.443	13140	4270	962,885
0.025	0.554	18760	6090	1,098,636
0.030	0.665	23560	7650	1,150,050
0.035	0.776	25160	8170	1,052,763
0.039	0.865	23780	7720	892,749



Remarks:
*Young's modulus is calculated using the secant modulus at each data point per Figure 2 (C) in ASMTM D 7012

Reviewed by: John Dailly

NCDOT No.: 105-03-0803